Vertical Ground Motion Effects on High-Rise Buildings

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WHY NOT VERTICAL GM?

1. Lower Energy Content Compared to Horizontal Components

2. A Large Safety Factor Against Gravity Loads Used in Design

WHAT TRIGGERS RESEARCHER'S INTEREST?

Northridge Earthquake

- January 17th, 1994
- California, USA
- Peak Vertical GM: 1.18g
- Peak V/H Ratio: 1.79

(Usually Assumed to be 2/3 in Design Code)

A LOT OF STRUCTURAL FAILURES

- High Mode Response
- Abrupt Changes of Stiffness and Strength in Elevation
- Lack of Capacity in High Storey
- Brittle Failure by Direct Compression
- Reduction of Shear and Flexural Strength and Ductility

SITE EVIDENCE OF VERTICAL GROUND MOTION



Symmetrical compressive column failure in a residential building.

Photograph courtesy of Earthquake Engineering Research Institute

SITE EVIDENCE OF VERTICAL GROUND MOTION



Brittle failure of the second floor of a building.

Photograph courtesy of Earthquake Engineering Research Institute

SITE EVIDENCE OF VERTICAL GROUND MOTION



Punching shear failure of upper floor in a Mall.

Photograph courtesy of Earthquake Engineering Research Institute

6 STORY BLDG, UNEQUAL COLS Time = 1.9944 Contours of Maximum Prin Stress max ipt. value min=-0.296699, at etcano 2642 max=0.331821, at years 6.51



Video by Dr. Gregory Szuladzinski, https://www.youtube.com/watch?v=DhM4HjceMfk

GM CHARACTERISTICS





Horizontal P-Wave Longer Period Lower Frequency Content Higher Overall Energy Content Vertical S-Wave Shorter Period Higher Frequency Content Lower Overall Energy Content

FACTORS AFFECTING VERTICAL GROUND MOTIONS

Strong Function of:

- Oscillator Period
- Source-to-Site Distance
- Local Soil Condition

Weak Function of:

- Earthquake Magnitude
- Type of Faulting

Niazi,M and Y. Bozognia . "Behavior of Vetical Ground Motion Parameters in the Near Field."

VERTICAL GM IN DESIGN CODE

"The vertical component of ground motion may be defined by scaling corresponding horizontal accelerations by a factor of two-thirds. Alternative factors may be used when substantiated by site-specific data. Where the Near-Source Factor, N_a , is greater than 1.0, site-specific vertical response spectra shall be used in lieu of the factor of two-thirds." The Near-Source Factor N_a is specified in Table 16-S of UBC-97 and is greater than 1.0 for the following cases:

- Seismic Source Type A when the closest distance to a known seismic source is < 10 km
- Seismic Source Type B when the closest distance to a known seismic source is < 5 km

ANALYTICAL STUDY OF A 42-STOREY BUILDING IN LOS ANGELES



Isotropic View

Plan View

ANALYTICAL MODEL OF A 42-STOREY BUILDING IN LOS ANGELES



GROUND MOTIONS SPECTRUM



V/H AND ARRIVAL TIME



STRUCTURAL RESPONSE



Ratio of the maximum floor acceleration in H1 direction

Ratio of the maximum inter-story drift ratio in H1 direction

STRUCTURAL RESPONSE





Ratio of the maximum shear force in the core wall in the H1 direction

Ratio of the maximum moment in the core wall in the H1 direction

STRUCTURAL RESPONSE





ANALYTICAL MODEL WITH THE DIAPHRAGM SYSTEM





AXIAL FORCE IN THE CORE WALL



CONCLUSION AND RECOMMENDATIONS

Problems:

- Axial force increases in structural components.
- Potential shear and moment capacity reduction.
- Ductility reduction.

Two simplified methods to more accurately scale vertical spectrum:

- 1. Shift the horizontal spectrum to shorter periods and reduce its amplitude to approximate the vertical spectrum.
- 2. Use V/H scaling factor.

